Attachment 1

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| Α | FOR | CONS | SENT | | | | BB | 28/8/ | 15 |
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WELLINGTON CITY COUNCIL REPLACEMENT OF BRIDGE No 6 O/S 330 SOUTH KARORI ROAD KARORI - WELLINGTON

DRAWING REGISTER FOR BRIDGE REPLACEMENT

| DRAWING | No | REV | DESCRIPTION |
|----------|-----|-----|---|
| E15-0296 | -00 | Α | DRAWING REGISTER FOR BRIDGE |
| E15-0296 | -01 | Α | GENERAL SPECIFICATIONS FOR BRIDGES |
| E15-0296 | -02 | Α | STANDARD DETAILS FOR REINFORCED CONCRETE FOR BRIDGES |
| E15-0296 | -04 | Α | GENERAL ARRANGEMENT OF NEW BRIDGE PLAN AND LONGITUDINAL PROFILE |
| E15-0296 | -05 | Α | GENERAL ARRANGEMENT OF NEW BRIDGE AND ROADWAY SECTIONS |
| E15-0296 | -06 | Α | EXISTING TOPO PLAN WITH SPOT LEVELS AROUND EXISTING BRIDGE |
| E15-0296 | -07 | Α | EXISTING BRIDGE PLANS WITH DEMOLITION WORKS |
| E15-0296 | -08 | Α | EXISTING BRIDGE SECTIONS WITH DEMOLITION WORKS |
| E15-0296 | -10 | Α | ALTERED BRIDGE PILING PLAN AND BRIDGE PLAN WITH NEW ABUTMENTS |
| E15-0296 | -11 | Α | NEW BRIDGE PRECAST PLAN AND BRIDGE PLAN WITH BARRIER BASES |
| E15-0296 | -12 | Α | NEW BRIDGE ABUTMENT ELEVATIONS AND PRECAST BRIDGE LONG SECTION |
| E15-0296 | -13 | Α | NEW BRIDGE ABUTMENT ELEVATION AND SECTIONS AT GRID 1 |
| E15-0296 | -14 | Α | NEW BRIDGE ABUTMENT ELEVATION AND SECTIONS AT GRID 2 |
| E15-0296 | -15 | Α | NEW BRIDGE PRECAST HOLLOWCORE UNIT PLANS WITH RECESS |
| E15-0296 | -16 | Α | NEW BRIDGE PRECAST HOLLOWCORE SECTIONS WITH REINFORCEMENT |
| E15-0296 | -17 | Α | NEW BRIDGE PRECAST SECTIONS AT END BLOCK AND AT DIAPHRAGMS |
| E15-0296 | -18 | Α | NEW BRIDGE PRECAST DECK WITH CONCRETE TOPPING AND BARRIER POSTS |

FOR CONSENT 28/08/2015

SpencerHolmes
engineers • surveyors planpers

Level 6 PO Box 588 Phone (04) 472-22 Email 8 Willis Street Wellington, NZ Fax (04) 471–2372 admin@spencerholmes.co.nz

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WELLINGTON CITY COUNCIL

ARCHITECT

PROJECT

BRIDGE No.6 O/S 330 SOUTH KARORI ROAD, WELLINGTON

DRAWING TITLE

DRAWING REGISTER FOR BRIDGE

| DESIGNED | APdG | □ CAD FILE | E15-0296-00 |
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| CHECKED | | □ DATE | AUG 2015 |
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PRELIMINARY AND GENERAL

THIS PAGE SHALL BE READ IN CONJUNCTION WITH THE STRUCTURAL SPECIFICATION DOCUMENT. 150296S01 STRUCTURAL SPECIFICATION.

REFER TO THE PRELIMINARY AND GENERAL CLAUSES OF THIS SPECIFICATION AND TO THE GENERAL CONDITIONS WHICH ARE EQUALLY BINDING TO ALL TRADES.

DRAWINGS ARE NOT TO BE SCALED OFF, IF IN DOUBT, SEEK CLARIFICATION. THE CONTRACTOR WILL BE RESPONSIBLE FOR THE ACCURATE SETTING OUT OF THE WORKS TRUE TO LINE AND LEVEL.

THE CONTRACTOR SHALL EMPLOY QUALIFIED STAFF TO DO THIS WORK.

WHERE NEW WORKS IS TO TIE INTO EXISTING STRUCTURE, THE CONTRACTOR IS RESPONSIBLE FOR MEASURING THE EXISTING STRUCTURE

THE EXISTING TOP OF ABUTMENTS ARE TO BE TRIMMED AS REQUIRED FOR NEW WORKS.

ALL MATERIALS AND WORKMANSHIP SHALL BE IN ACCORDANCE WITH THE CURRENT CODES OF PRACTICE EXCEPT WHERE VARIED BY THE SPECIFICATION AND/OR DRAWINGS.

CONSTRUCTION METHODOLOGY

THE CONTRACTOR SHALL HAVE A CONSTRUCTION METHODOLOGY PRIOR TO BEGINNING WORKS ON-SITE. THE CONTRACTOR SHALL PROVIDE THE CONSTRUCTION METHODOLOGY TO SPENCER HOLMES LIMITED FOR RECORD PURPOSES

THE CONTRACTOR AND SUBCONTRACTORS SHALL FOLLOW THE CONSTRUCTION METHODOLOGY.

TEMPORARY WORKS DESIGN

THE TEMPORARY WORKS CONSISTS OF THE PREPARATION OF PROGRAMMES, METHODOLOGY STATEMENTS AND "FALSEWORK" DESIGN THAT DEFINE THE METHOD AND SEQUENCE IN WHICH THE BUILDING OR STRUCTURE IS TO BE BUILT. IN THIS CONTEXT, FALSEWORK REFERS TO ALL STAGING, TEMPORARY PROPPING AND BRACING, SCAFFOLDING AND ACCESS, THAT IS, THAT THERE IS AT LEAST ONE METHOD AND SEQUENCE THAT CAN BE FOLLOWED TO CONSTRUCT THE DESIGN, SHOP DRAWING, SHORING, CRANAGE, SITE ACCESS AND THE LIKE.

THE CONTRACTOR SHALL BE RESPONSIBLE FOR PROVIDING ALL APPROPRIATE TEMPORARY WORKS FOR THE PROJECT TO BE UNDERTAKEN AND COMPLETED IN A SAFE AND SECURE MANNER.

SUPPORT AND BRACE THE EXISTING STRUCTURE DURING THE CUTTING OF NEW OPENINGS OR THE REPLACEMENT OF STRUCTURAL PARTS, PREVENT DEBRIS FROM OVERLOADING ANY PART OF THE STRUCTURE. DO NOT REMOVE SUPPORTS UNTIL THE NEW WORK IS STRONG ENOUGH TO SUPPORT THE EXISTING STRUCTURE. ENSURE ALL WORK REMAINS STRUCTURALLY STABLE AND SOUND

DRAWINGS

THE DRAWINGS INDICATE THE GENERAL ARRANGEMENT AND DIMENSIONS HAVE BEEN MADE TO SCALE. WHERE DIMENSIONS HAVE NOT BEEN GIVEN, SCALED DIMENSIONS SHALL NOT BE USED. CONFIRM ALL DIMENSIONS WITH ON-SITE MEASUREMENTS.

IF ANY DISCREPANCIES ARE FOUND WITHIN THE DRAWINGS REFER TO THE ENGINEER FOR INSTRUCTIONS BEFORE PROCEEDING.

SCOPE OF WORKS

THE WORKS OF THIS CONTRACT INCLUDE THE SUPPLY OF ALL LABOUR. MATERIALS, PLANT AND EQUIPMENT ASSOCIATED WITH THE REPLACEMENT OF SOUTH KARORI BRIDGE No 6. LOCATED OUTSIDE 330 SOUTH KARORI ROAD, KARORI, WELLINGTON

THE CONTRACT FOR THE STRENGTHENING WORKS SHALL INCLUDE THE PARTIAL DEMOLITION OF THE EXISTING BRIDGE, CONSTRUCTION OF A NEW BRIDGE AND ASSOCIATED CIVIL WORKS.

DESIGN BASIS

DESIGN STANDARD

APPENDIX A LIGHTLY TRAFFICKED RURAL BRIDGES, NEW ZEALAND TRANSPORTATION AUTHORITY BRIDGE MANUAL (UP TO AND INCLUDING AMENDMENTS TO SEPTEMBER 2014).

TRAFFIC LOAD

0 85 HN

SLS 1a LOAD FACTOR FOR LIVE LOAD OF 1.35.

SINGLE LANE

SEISMIC DESIGN

DESIGN LOAD IN ACCORDANCE WITH NZS 1705 WITH Co = Ch(To) Z Ru Sp = 1.00x 0.4x1.3x0.9 = 0.468. SOIL CLASS B

WIND DESIGN

DESIGN LOAD IN ACCORDANCE WITH AS/NZS 1170.2 WITH SITE WINDSPEED Vsit = 53m/s.

BARRIER DESIGN

ON BRIDGE TEST LEVEL 3.

PRESTRESSED GIRDERS

PRESTRESSED GIRDERS ARE DESIGNED TO CLASS C AS DEFINED BY NZS 3101.

CONSTRUCTION MONITORING

THE PRIMARY RESPONSIBILITY FOR COMPLETING THE CONTRACT WORKS IN ACCORDANCE WITH THE REQUIREMENTS OF THE PLANS AND SPECIFICATIONS IS THE CONTRACTOR'S. SPENCER HOLMES LIMITED HAVE BEEN ENGAGED TO UNDERTAKE CONSTRUCTION MONITORING TO LEVEL CM3 IN ACCORDANCE WITH ACENZ CONDITIONS OF ENGAGEMENT OF THE STRUCTURAL ASPECTS WHICH HAVE BEEN SPECIFICALLY DESIGNED.

CONTACTS: SPENCER HOLMES LIMITED - ANDRE DE GROOT. PHONE 04 472 2261

INSPECTION SCHEDULE

THE FOLLOWING SCHEDULE OF ON-SITE INSPECTIONS IS TO BE MET IN ORDER TO MEET THE REQUIRED LEVEL OF CONSTRUCTION MONITORING, AND TO ENSURE THE INTENT OF THE DESIGN IS MET:

TEMPORARY PROPPING. DEMOLITION OF EXISTING BRIDGE SUPERSTRUCTURE. MODIFICATION OF EXISTING ABUTMENTS. INSTALLATION OF PILES AND ROCK ANCHORS EXCAVATION OF ABUTMENTS AND PLACING OF REINFORCING. REINFORCING FOR HOLLOWCORE BRIDGE BEAMS. REINFORCING FOR DECK POUR. INSTALLATION OF GUARDRAILS PAVEMENT RECONSTRUCTION HOLDPOINT PRIOR TO SEALING. ASPHALT.

COMPLETION

THE CONTRACTOR SHALL PROVIDE SPENCER HOLMES LIMITED WITH A SIXTH SCHEDULE FORM OF PRODUCER STATEMENT - CONSTRUCTION IN ACCORDANCE WITH NZS 3910 ON COMPLETION OF THE STRUCTURAL WORKS.

SPENCER HOLMES LIMITED WILL PROVIDE A PRODUCER STATEMENT PS4 CONSTRUCTION REVIEW, ALONG WITH COPIES OF THE SITE INSPECTION REPORTS. ON COMPLETION OF THE STRUCTURAL WORKS.



Level 6 PO Box 588 Phone (04) 472-2261 Email

Wellington, NZ Fax (04) 471-2372

WELLINGTON CITY COUNCIL

ARCHITECT

PROJECT

BRIDGE No.6 O/S 330 SOUTH KARORI ROAD. WELLINGTON

DRAWING TITLE

GENERAL SPECIFICATION FOR BRIDGES

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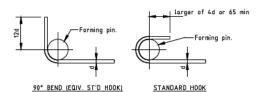
BENDS AND ANCHORAGES

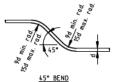
MINIMUM DIAMETER OF BENDS (INSIDE FACE)

| YEILD STRENGTH | BAR DIA. | MAIN REINFORCEMENT |
|----------------|----------|--------------------|
| 300 OR 500 | 6 - 20 | 5d |
| 300 OR 500 | 25 - 40 | 6d |

| YEILD STRENGTH | BAR DIA. | STIRRUPS AND TIES | | |
|----------------|----------|-------------------|----------|--|
| TEILU SIKENGIN | d | PLAIN | DEFORMED | |
| 300 OR 500 | 6 - 20 | 2d | 4d | |
| 300 OR 500 | 25 - 40 | 3d | 6d | |

NOTE :-d = DIAMETER OF BAR BEING BENT.





STARTER BARS PARTIALLY EMBEDDED IN CONCRETE SHALL NOT BE BENT OVER UNLESS SPECIFICALLY

STIRRUPS FOR WALLS

 $\frac{\text{NOTE}}{1:}$:- 1: First wall stirrup positioned 50 mm from the column face.

2 : All to be pigin round bars grade 300E.



STIRRUPS





COVERS TO REINFORCEMENT

| | IN CONTACT | WITH GROUND | ABOVE GROUND LEVEL | | |
|--------------------------|------------------------|--|-----------------------|------------------|--|
| ITEM | CAST AGAINST GROUND | AGAINST APPROVED BOXING, SITE CONCRETE OR D.P.M. | EXPOSED TO WEATHER | AGAINST VOIDS | |
| CAST INSITU | 75 | 50 | 40 | 30 | |
| PRECAST HOLLOWCORE UNITS | - | - | 35 | 30 | |

USE THESE COVERS FOR ALL REINFORCING UNLESS NOTED OTHERWISE ON THE DRAWINGS.

CONCRETE STRENGTHS

| ITEM | STRENGTH × |
|--------------------------|------------|
| SITE CONCRETE | 10 MPa |
| CAST INSITU | 40 MPa |
| PRECAST HOLLOWCORE UNITS | 50 MPa |

* CONCRETE STRENGTHS SHALL BE COMPRESSIVE CYLINDER STRENGTHS TESTED AT 28 DAYS

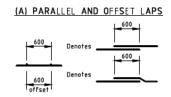
SURFACE FINISHES

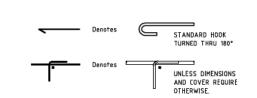
(B) HOOKS AND BENDS

| ITEM | FINISH × |
|-------------------------------|----------|
| EXPOSED CONCRETE | F5 |
| BURIED CONCRETE | F2/U2 |
| TOP SURFACE OF DECK SLAB | U6 ×× |
| TOP SURFACE OF UNFORMED ITEMS | U3 ××× |

- SURFACE FINISHES IN ACCORDANCE WITH NZS 3114
- ** BROOM DIRECTION SHALL BE TRANVERSE TO TRAFFIC DIRECTION
- *** EXCEPT CONSTRUCTION JOINTS WHICH SHALL BE TYPE B ACCORDING TO NZS 3109

PICTORIAL REPRESENTATION OF REINFORCING BARS ON DRAWINGS.





REINFORCEMENT NOTATIONS:
30-012-250 DENOTES 30 NO. 12mm DIA. DEFORMED BARS AT 250 CRS.
30-R12-250 DENOTES 30 NO. 12mm DIA. PLAIN ROUND BARS AT 250 CRS.
30-HD12 250 DENOTES 30 NO. 12mm DIA. GRADE 500 DEFORMED BARS AT 250 CRS. For lap lengths refer table OFFSET LAPS

NOTE:-WHERE LAP POSITIONS ARE NOT SHOWN ON THE DRAWINGS, LAPS IN WALL AND SLAB REINFOCEMENT ARE TO BE LAPPED AT RANDOM IN A STAGGERED PATTERN, IN POSITIONS OF LOW STRESS.

WHEN TWO BARS OF DIFFERING SIZES LAP, THE LAP LENGTH OF THE SMALLER OF THE TWO BARS SHALL APPLY.

50 max.

WELDED WIRE MESH

LAP LENGTHS

500 GRADE REINFORCING

| MINIMUM LAP LENGTHS FOR 40 MPa CONCRETE | | | | | |
|---|-----------|------------------------------------|-----------------|--|--|
| BAR SIZE | ANCHORAGE | BEAMS AND COLUMNS TYPICAL STEEL | BEAMS TOP STEEL | | |
| 10 | 400 | 400 | 550 | | |
| 12 | 500 | 500 | 650 | | |
| 16 | 650 | 650 | 850 | | |
| 20 | 800 | 800 | 1050 | | |
| 25 | 1000 | 1000 | 1300 | | |
| 32 | 1300 | 1300 | 1650 | | |

USE THESE LAP LENGTHS FOR ALL REINFORCING UNLESS NOTED OTHERWISE ON THE DRAWINGS.

GENERAL NOTES

UNLESS OTHERWISE SPECIFIED OR DETAILED ON THE DRAWINGS, THESE NOTES AND ONLESS OTHERWISE SPECIFIED OR DETAILED ON THE DRAWINGS, THESE NUTES AND DETAILS SHALL APPLY, BUT INCLUSION OF THIS DRAWING IN THE CONTRACT SET DOES NOT IMPLY THAT ALL DETAILS DCCUR IN THIS CONTRACT. THE CONTRACTOR MUST NOT POUR CONCRETE BEFORE THE REINFORCING HAS BEEN INSPECTED IN PLACE BY THE CONSULTING ENGINEER WHO SHALL BE GIVEN A MIN. OF 24 HOURS NOTICE OF POURING.

WHERE ALTERNATIVES ARE SHOWN ON THIS DRAWING THE CONTRACTOR MUST OBTAIN APPROVAL FOR THE USE OF ONE OR ANY OF THESE DETAILS AND SHALL NOT CHANGE FROM THIS APPROVAL WITHOUT FURTHER APPROVAL.

THE STRUCTURAL DRAWINGS ARE TO BE READ IN CONJUNCTION WITH THE DRAWINGS OF OTHER SPECIALISTS I.E. ARCHITECTS, MECHANICAL, BUILDING SERVICES, ELECTRICAL ETC.

FORM FILLETS TO ALL CORNERS OF CONCRETE WORK, FITHER INSITU OR PRECAST, AS REQUIRED BY THE SPECIFICATION UNLESS NOTED OTHERWISE

REINFORCEMENT

R and D BARS ARE GRADE 300E COMPLYING WITH AS/NZS 4671.

HR, HD AND RB BARS ARE GRADE 500 E MA (MICRO-ALLOYED) COMPLYING WITH AS/NZS 4671.

WELDED WIRE REINFORCEMENT SHALL BE CLASS E-DUCTILE MESH ACCORDING TO AS/NZS 4571.

COUPLERS

ALL COUPLERS SHALL MEET THE REQUIREMENTS OF THE NZTA BRIDGE MANUAL.

SUBMIT PROPSED COUPLERS TO THE DESIGN ENGINEER FOR APPROVAL

ABBREVIATIONS

| ALT. | = | ALTERNATE |
|--------|---|----------------------------------|
| B | = | BOTTOM |
| ŗ | = | COVER |
| č.J. | = | CONSRTUCTION JOINT |
| CL | = | CENTRE LINE |
| COL. | - | COLUMN |
| CRS | = | CENTERS |
| d | = | NOMINAL BAR DIAMETER |
| D | = | DEFORMED BAR GRADE 300 |
| D.P.C. | = | DAMP PROOF COURSE |
| D.P.M. | = | DAMP PROOF MEMBRANE |
| E.F. | = | EACH FACE |
| E.J. | = | EXPANSION JOINT |
| E.W. | = | EACH WAY |
| E.E. | = | FAR FACE |
| FD'N | = | FOUNDATION |
| HD | = | DEFORMED BAR GRADE 500F MICROALI |
| HR | = | PLAIN ROUND BAR GRADE 500E MICRO |
| i.J. | = | ISOLATION JOINT |
| L.A.R. | = | LAP AT RANDOM |
| MAX | = | MAXIMUM |
| MIN | - | MINIMUM |
| N.F. | = | NEAR FACE |
| N.T.S | = | NOT TO SCALE |
| PC | = | PRECAST CONCRETE |
| R | = | PLAIN ROUND BAR GRADE 300E |
| RL | = | REDUCED LEVEL |
| RB | = | REIDBAR GRADE 500E MICROALLOYED |
| S.J. | = | TAIOL AWAZ |
| STRP | = | STIRRUP |
| T | = | TOP |
| TYP | = | TYPICAL |
| U.N.O. | = | UNLESS NOTED OTHERWISE |
| · | | SHEEDS HOLES STREET |

0 **SpencerHolmes** \otimes $\overline{\mathbb{Q}}$ Level 6 PO Box 588 Phone (04) 472-2261 Email Wellington, NZ Fax (04) 471-2372 WELLINGTON CITY COUNCIL \otimes (J ARCHITECT BA M

BRIDGE No.6 O/S 330 SOUTH KARORI ROAD, WELLINGTON

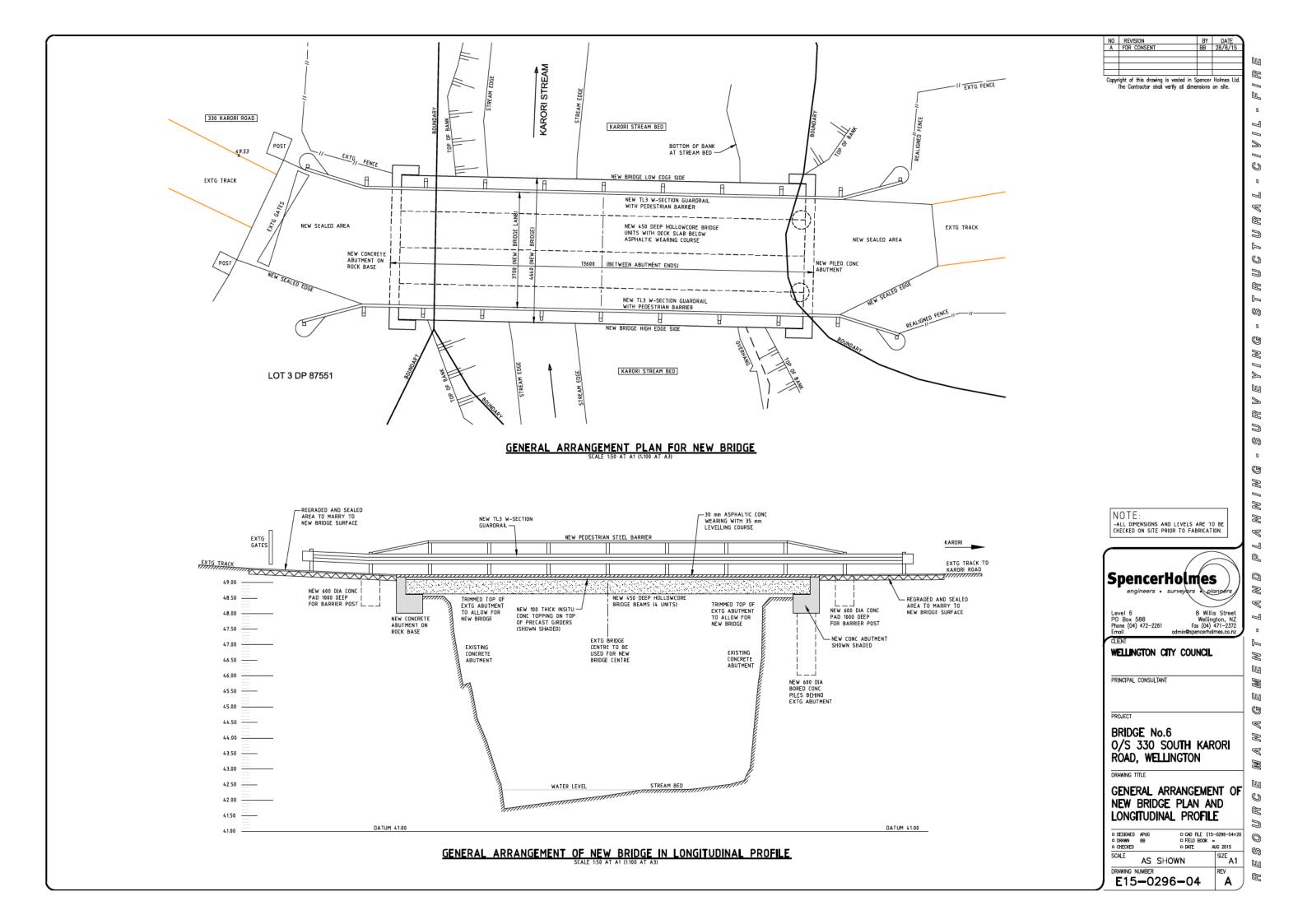
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PROJECT

STANDARD DETAILS FOR REINFORCED CONCRETE FOR BRIDGES

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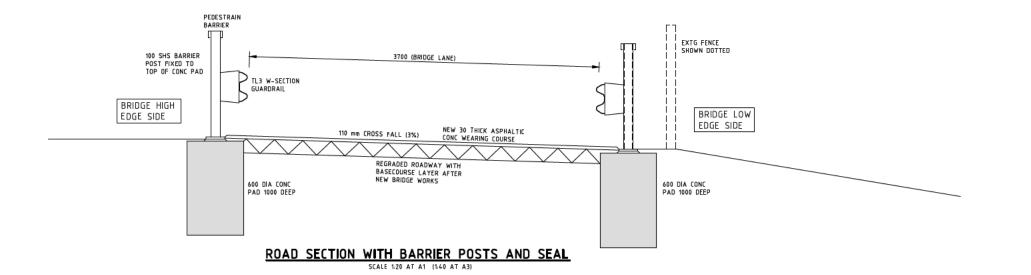
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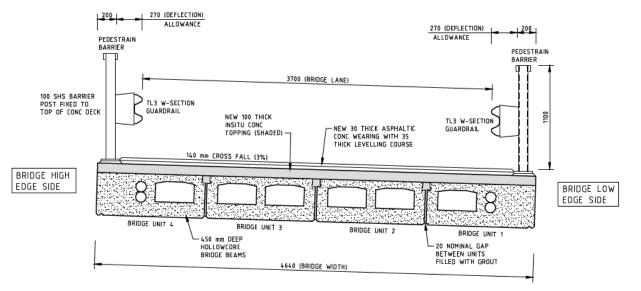
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BRIDGE SECTION WITH BARRIER POSTS AND SEAL

TALE 1:20 AT A1 (1:40 AT A3)

NOTE:
-ALL DIMENSIONS AND LEVELS ARE TO BE CHECKED ON SITE PRIOR TO FABRICATION.

SpencerHolmes engineers · surveyors · plans

Level 6 PO Box 588 Phone (04) 472-2261 Email 8 Willis Street Wellington, NZ Fax (04) 471-2372 dmin@spencerholmes.co.nz

WELLINGTON CITY COUNCIL

PRINCIPAL CONSULTANT

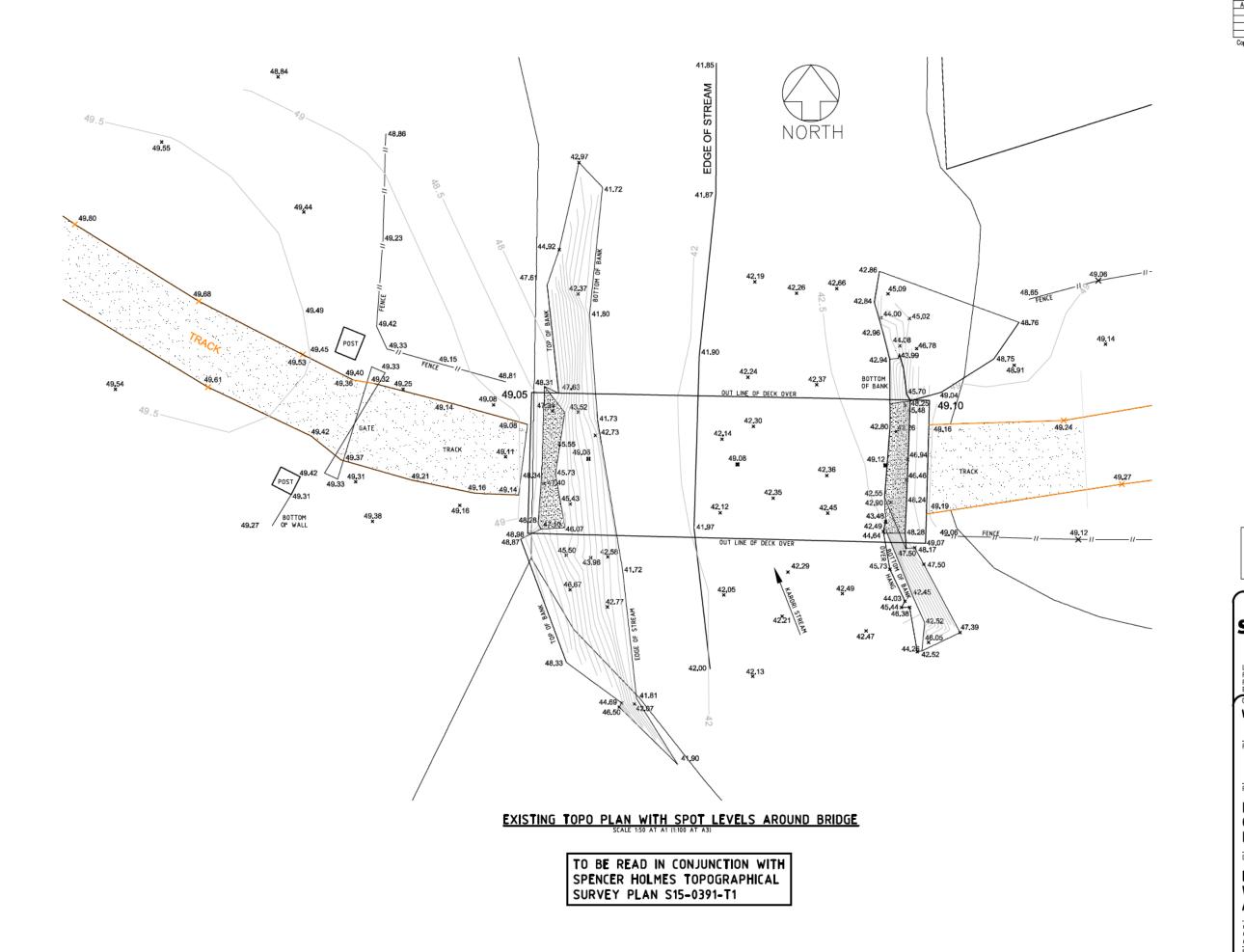
PROJECT

BRIDGE No.6 O/S 330 SOUTH KARORI ROAD, WELLINGTON

DRAWING TITLE

GENERAL ARRANGEMENT OF NEW BRIDGE AND ROADWAY SECTIONS

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The Contractor shall verify all dimensions on site.

NOTE:
-EXISTING STRUCTURE IS ASSUMED FROM VISUAL INSPECTION ONLY.
-ALL DIMENSIONS AND LEVELS ARE TO BE CHECKED ON SITE PRIOR TO FABRICATION.

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WELLINGTON CITY COUNCIL

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PROJECT

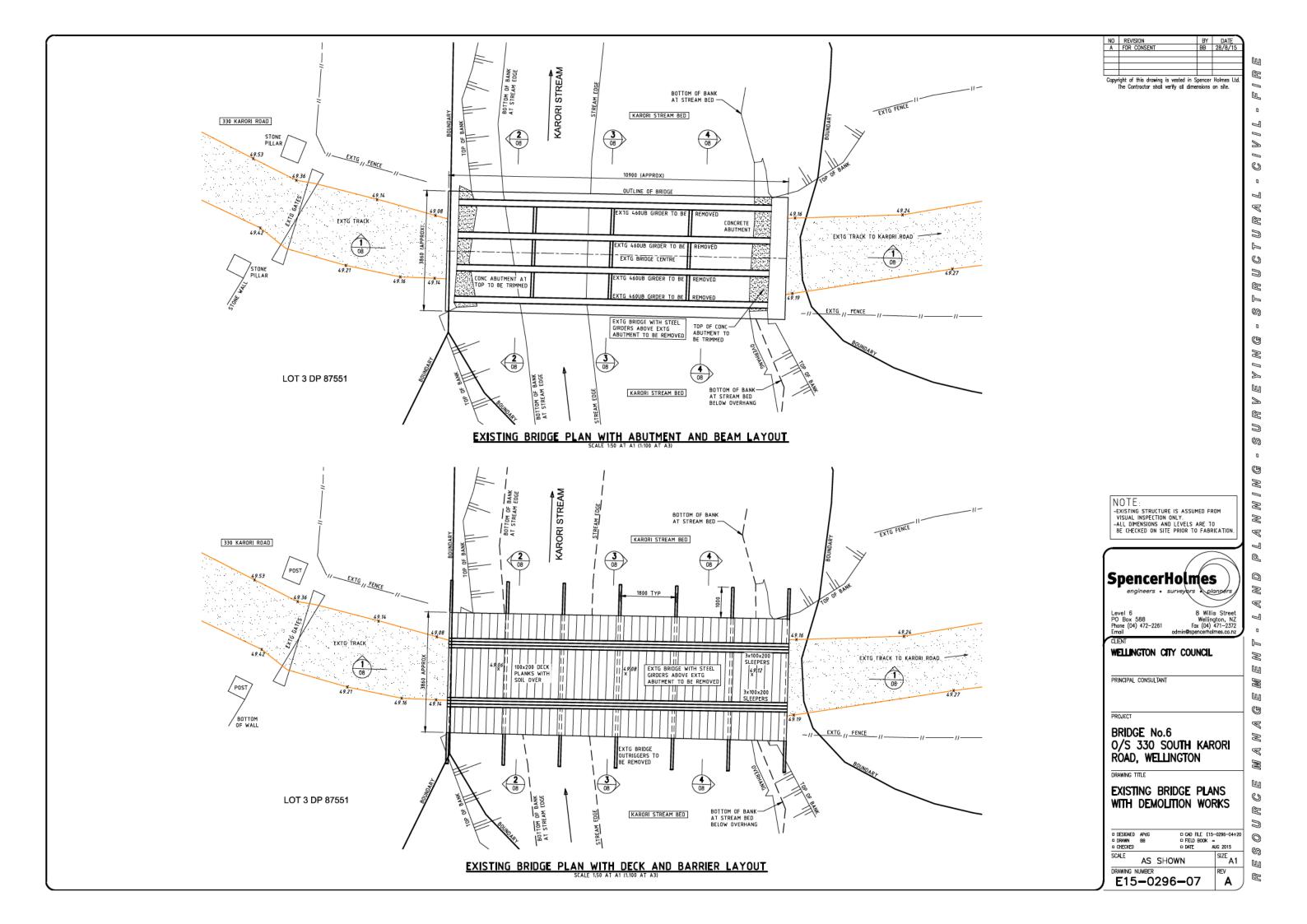
BRIDGE No.6 O/S 330 SOUTH KARORI ROAD, WELLINGTON

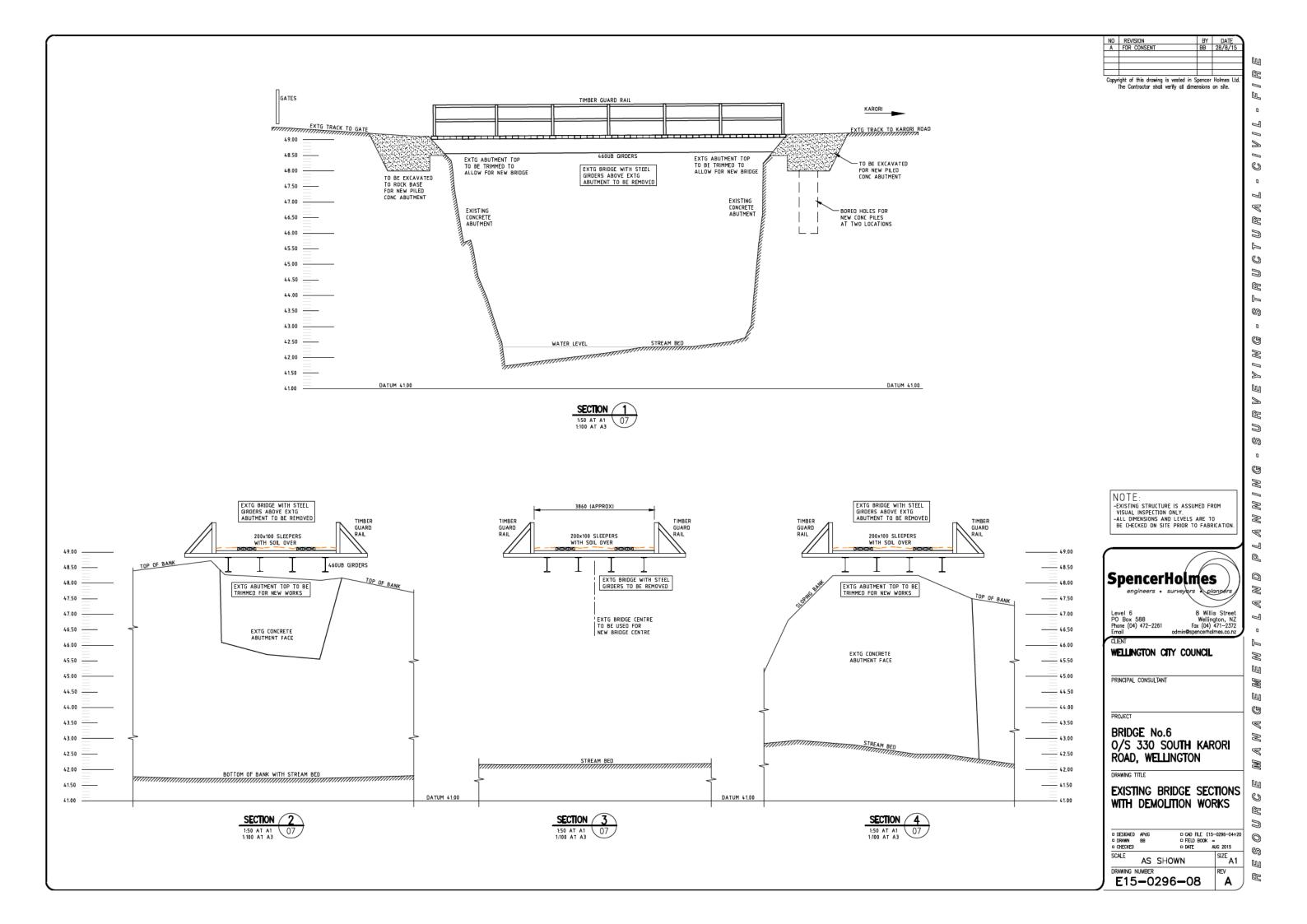
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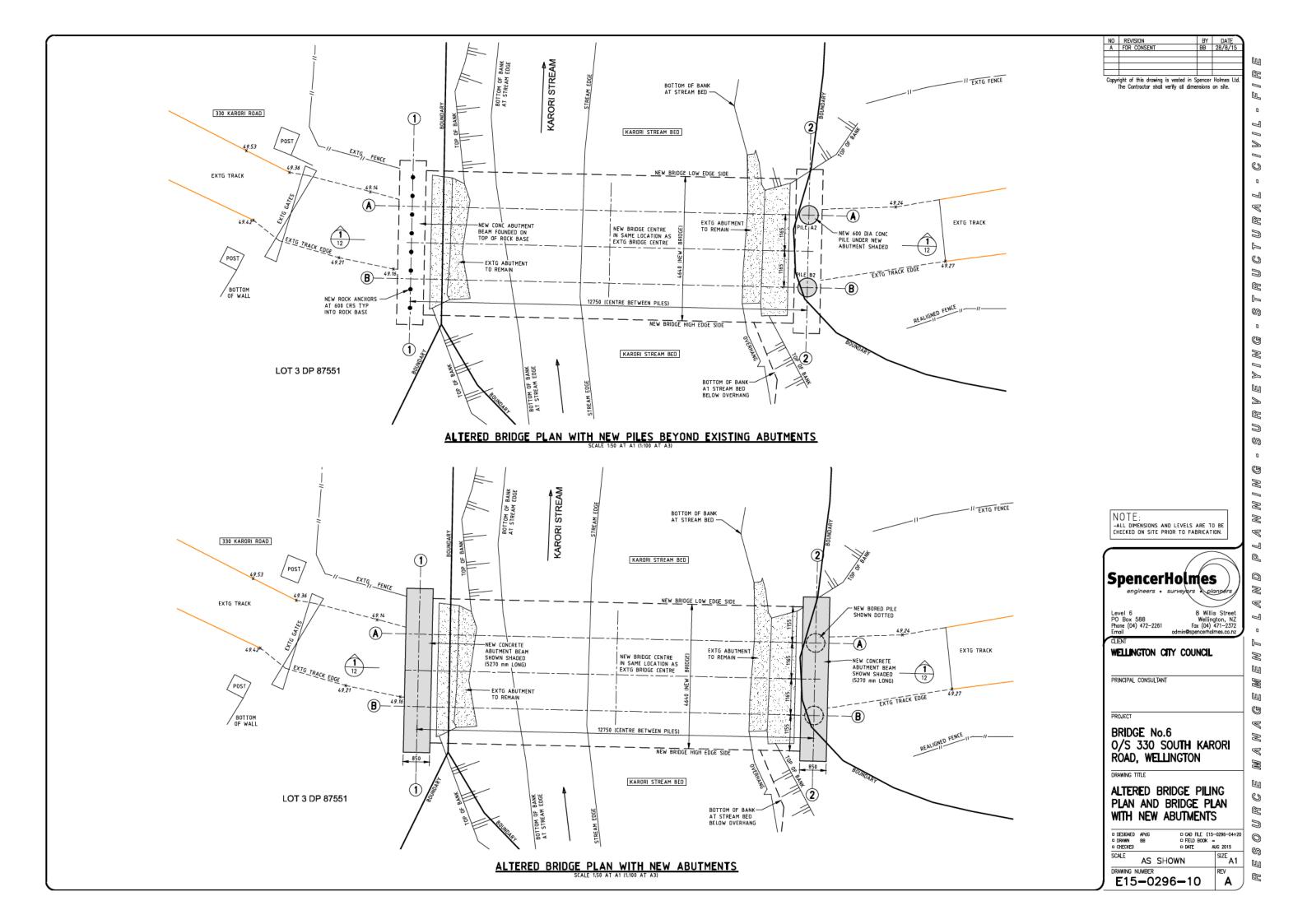
EXISTING TOPO PLAN WITH SPOT LEVELS AROUND BRIDGE

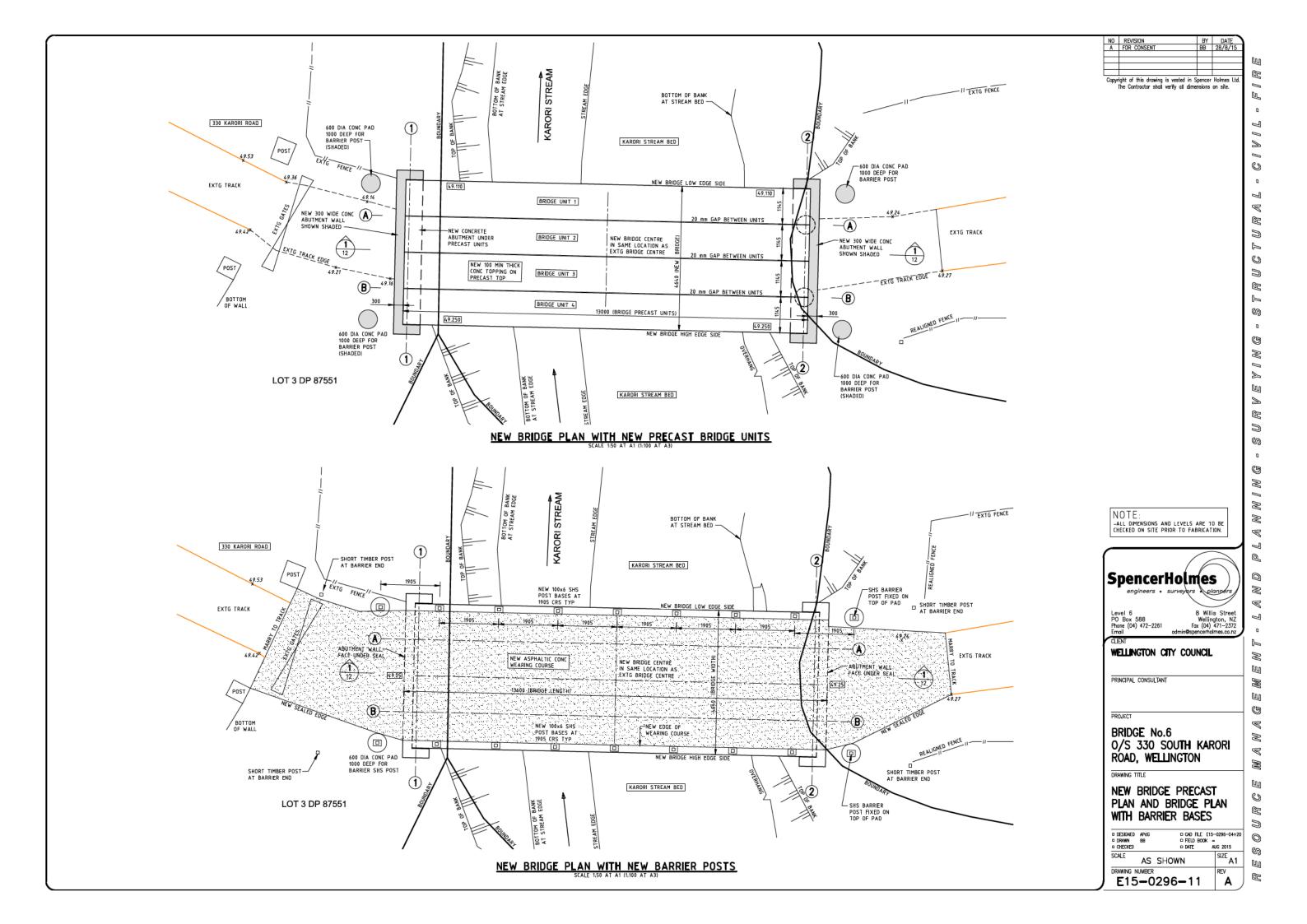
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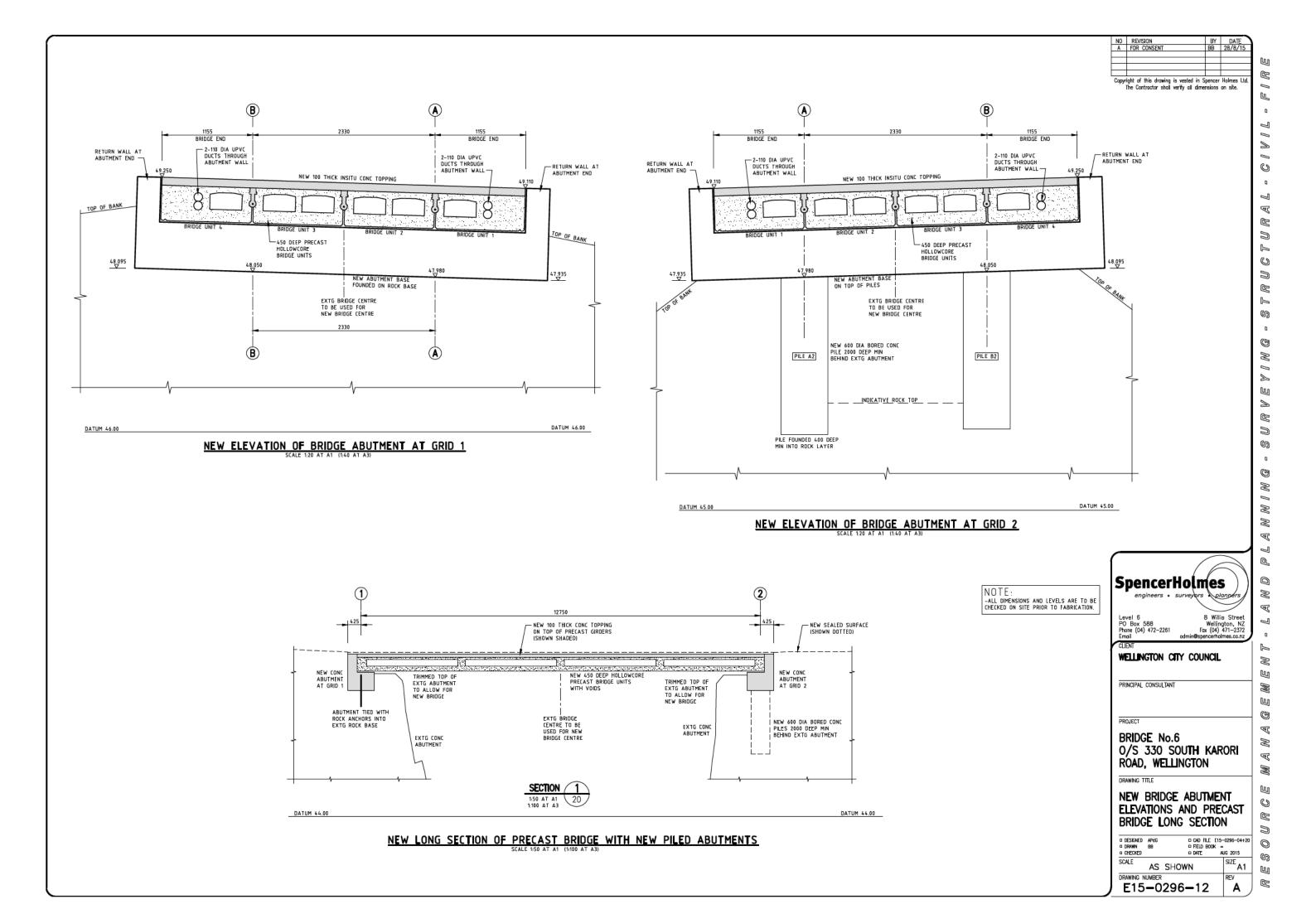
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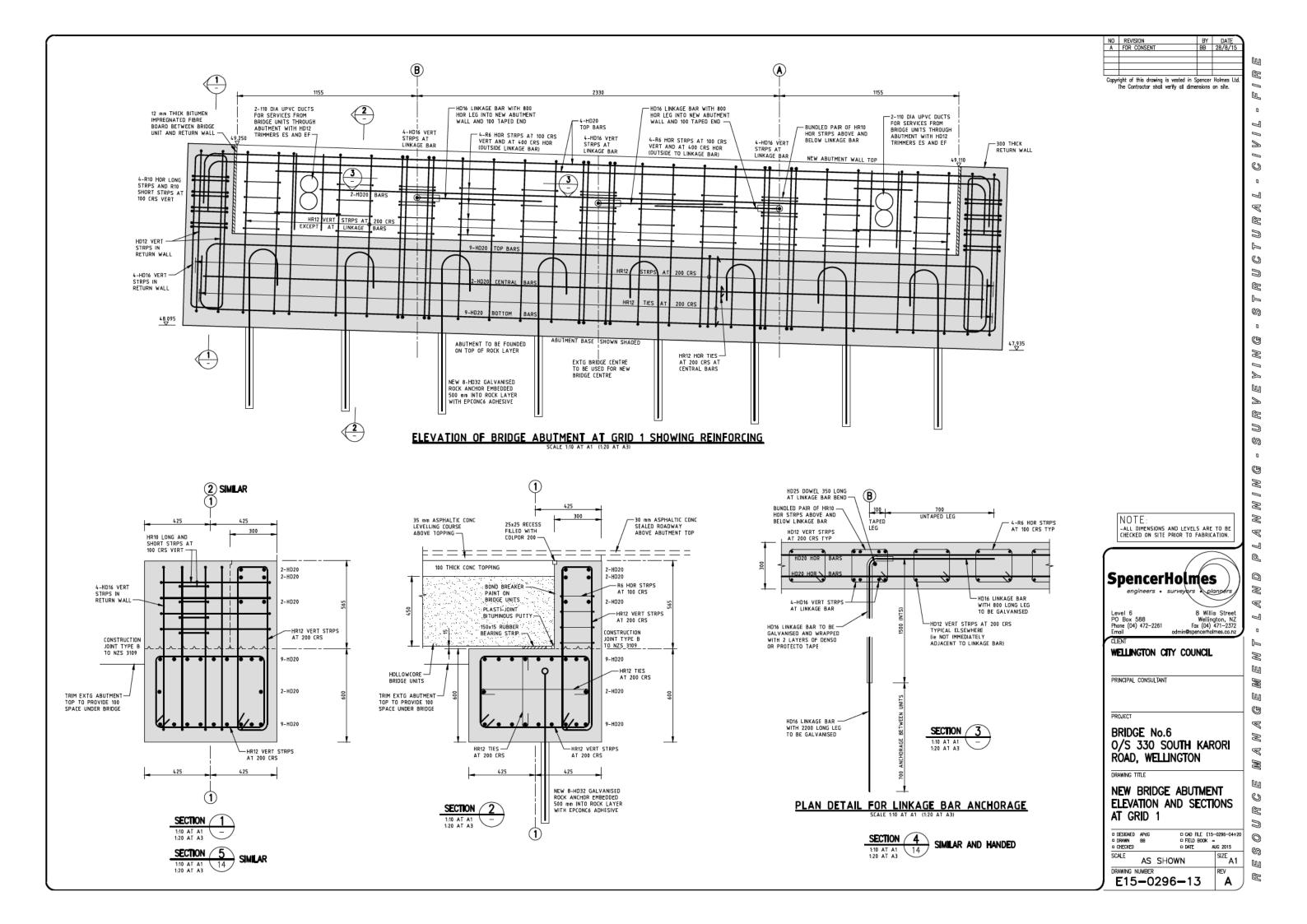


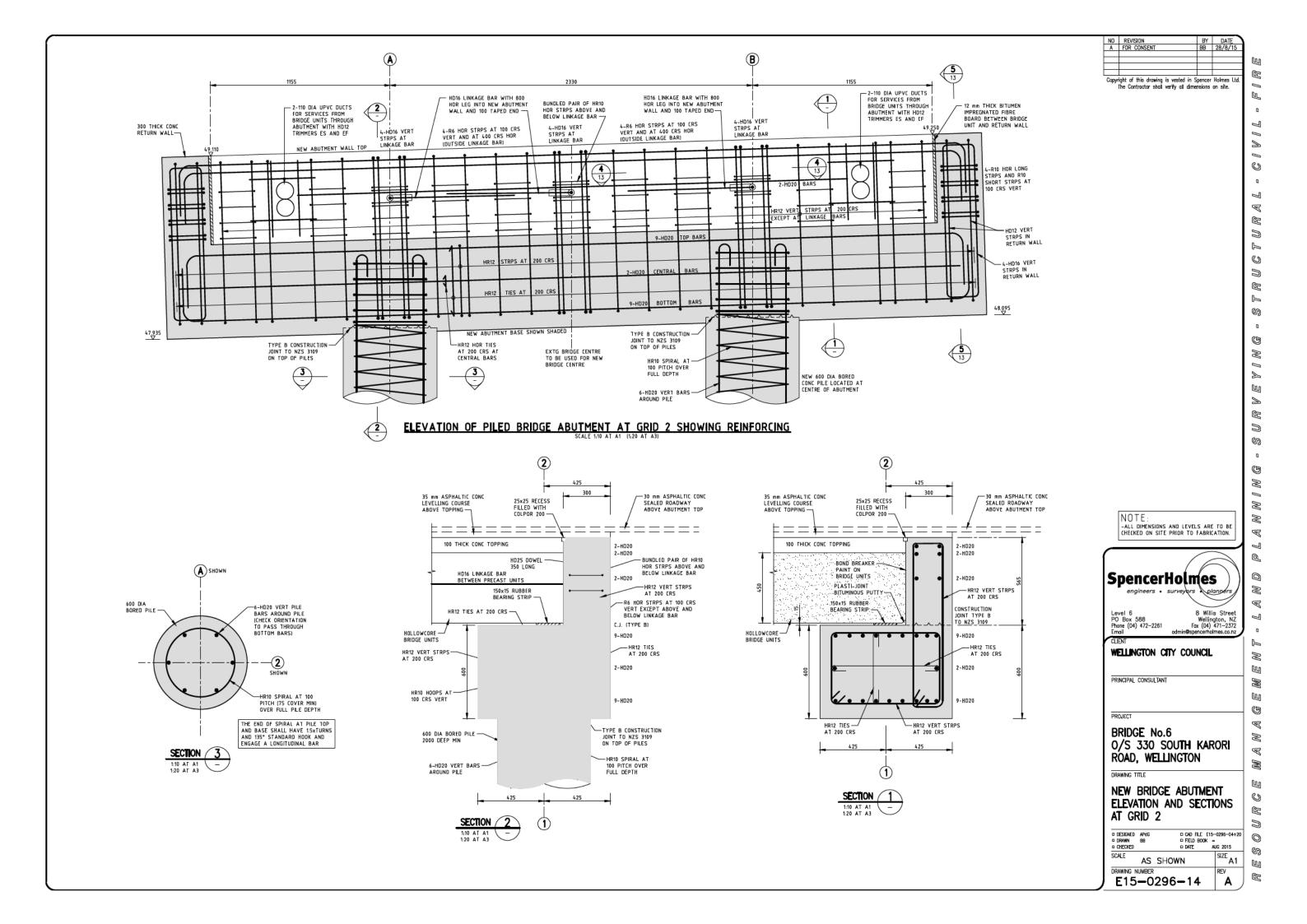












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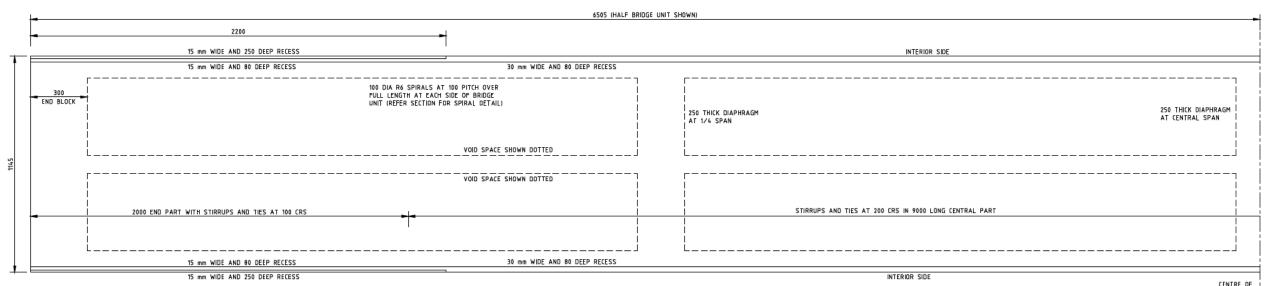
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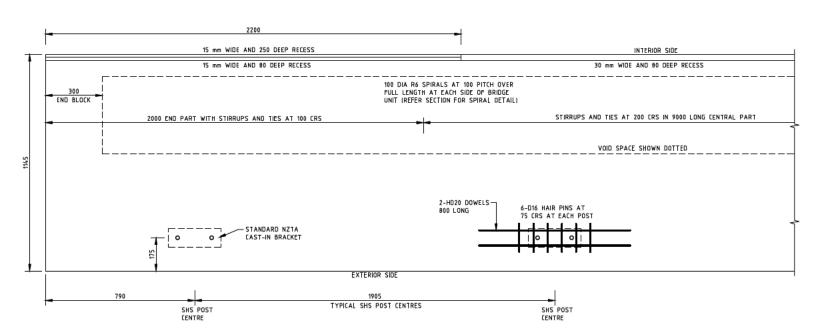
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GEOMETRY PLAN OF BRIDGE UNIT 3 BETWEEN END AND UNIT CENTRE

END OF BRIDGE UNIT 2 SIMILAR



GEOMETRY PLAN AT END OF BRIDGE UNIT 4

END OF BRIDGE UNIT 1 HANDED

REFER TO DRAWING E15-0296-16 FOR PRECAST HOLLOWCORE SECTIONS WITH REINFORCING

NOTES FOR HOLLOWCORE BEAM REINFORCEMENT

STRANDS

RELAX 2 (LOW RELAXATION SUPER STRAND 12.9 DIA WITH AN AREA OF 100 mm2 MINIMUM ULTIMATE TENSILE STRENGTH OF 186 KN INITIAL PRESTRESS FORCE OF 130 KN ALL STRANDS SHALL COMPLY WITH AS/NZS 4672

STRANDS SHALL BE RELEASED SLOWLY AND AFTER RELEASE SHALL BE CUT AND GROUND FLUSH WITH THE CONCRETE AT THE FND OF THE UNIT.

A THICK COATING OF HIGH BUILD EPOXY PAINT SHALL BE APPLIED AFTER GRINDING BEFORE THE UNIT LEAVES THE CASTING YARD.

EACH END OF HOLLOWCORE BRIDGE BEAMS SHALL BE LABELLED EG BEAM 1 EAST END.

CONCRETE STRENGTH SHALL REACH MINIMUM COMPRESSIVE STRENGTH OF 30 MPa AT TRANSFER.

GIRDERS MAY BE PLACED AT 14 DAYS.

LIFTING POINTS SHALL BE DESIGNED BY THE CONSTRUCTOR. CAST THE CENTRAL BEAMS FIRST AND MOVE OUTWARDS.

SUBMIT TO DESIGN ENGINEER THE PROPOSED CURING METHODOLOGY FOR APPROVAL

INCLUDE EXPECTED INTERNAL AND EXTERNAL TEMPERATURE RISE.
THE CONCRETE POUR SHALL BE COMPLETED BEFORE THE HEATING OF FORMS COMMENCES.

| ESTIMATED BEAM PROPERTIES | |
|---|-------|
| MASS (TONNES) | 11.7 |
| ELASTIC SHORTENING AFTER STRAND RELEASE | 6 |
| HOG AFTER STRAND RELEASE | 9 |
| HOG AFTER 14 DAYS | 15 |
| HOG AFTER 60 DAYS | 22 |
| DEFLECTION DUE TO DECK POUR | 5 |
| CAST LENGTH IN mm (XX) | 13010 |

XX = INCLUDES ALLOWANCES FOR ELASTIC SHORTENING, CREEP AND SHRINKAGE TO 14 DAYS NOTE:

-ALL DIMENSIONS AND LEVELS ARE TO BE CHECKED ON SITE PRIOR TO FABRICATION.



Level 6 PO Box 588 Phone (04) 472-2261 Emgil

Wellington, NZ Fax (04) 471-2372

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BRIDGE No.6 O/S 330 SOUTH KARORI ROAD, WELLINGTON

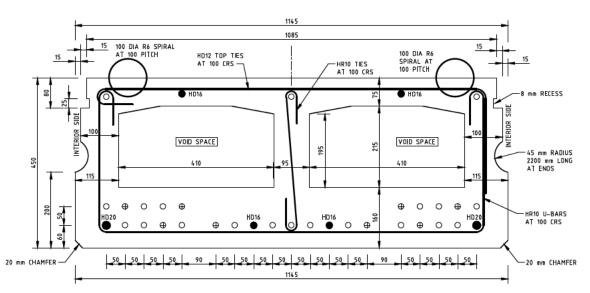
DRAWING TITLE

NEW BRIDGE PRECAST HOLLOWCORE UNIT PLANS WITH RECESS

| Designed Drawn Checked | APdG BB | □ CAD FILE E15 □ FIELD BOOK □ DATE | |
|--|------------|--|------|
| SCALE | | | SIZE |

AS SHOWN ٦A1 DRAWING NUMBER E15-0296-15

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GEOMETRY SECTION OF BRIDGE UNIT 3 FOR 2m END PART O 12.9 DIA STRAND

1085 100 DIA R6 SPIRAL AT 100 PITCH 100 DIA R6 SPIRAL AT 100 PITCH VOID SPACE VOID SPACE **000** \oplus \circ \circ \oplus ⊕ O ⊕ O AT 200 CRS 50 50 50 50 90 50 50 50 50 50 50 50 50 50 50 20 mm CHAMFER --20 mm CHAMFER

GEOMETRY SECTION OF BRIDGE UNIT 3 CENTRAL PART

BRIDGE UNIT 2 SIMILAR

BRIDGE UNIT 2 SIMILAR

20 mm CHAMFER -

40 mm WIDE— DRIP LINE

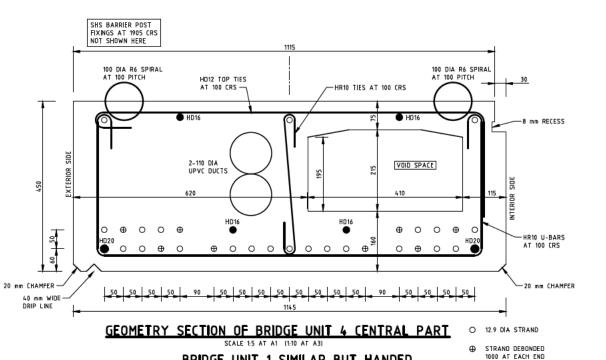
SHS BARRIER POST ARE AT 1905 CRS STANDARD NZTA 100 DIA R6 SPIRAL NOT SHOWN HERE FOR CLARITY 100 DIA R6 SPIRAL AT 100 PITCH HD12 TOP TIES AT 100 CRS — 2-HD20 -HR10 TIES AT 100 CRS 800 LONG -8 mm RECESS AT 75 CRS AT EACH POST VOID SPACE 2200 mm LONG AT ENDS **B** O O **9009** 0 0 0 **#** 0 0 0 о о о в

GEOMETRY SECTION OF BRIDGE UNIT 4 FOR 2m END PART O 12.9 DIA STRAND

BRIDGE UNIT 1 SIMILAR BUT HANDED

REFER TO DRAWING E15-0296-15 FOR NOTES FOR HOLLOWCORE BEAM REINFORCEMENT

⊕ STRAND DEBONDED



BRIDGE UNIT 1 SIMILAR BUT HANDED

-ALL DIMENSIONS AND LEVELS ARE TO BE CHECKED ON SITE PRIOR TO FABRICATION.

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DRAWING TITLE

NEW BRIDGE PRECAST HOLLOWCORE SECTIONS WITH REINFORCEMENT

SIZE A1 AS SHOWN DRAWING NUMBER E15-0296-16

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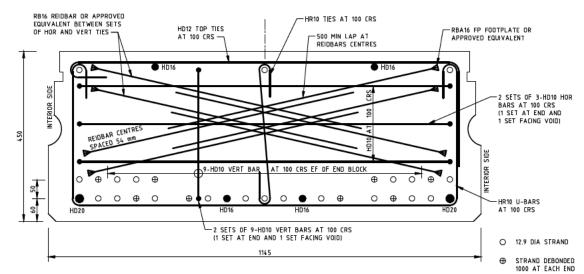
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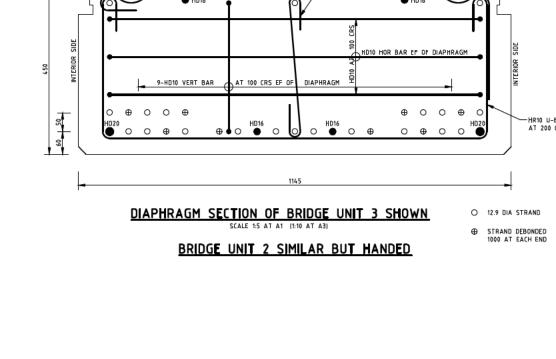
100 DIA R6 SPIRAL AT 100 PITCH

-HR10 TIES AT 200 CRS



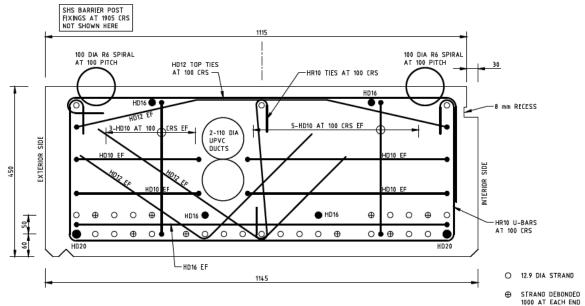
REINFORCING SECTION AT END BLOCK OF BRIDGE UNIT 3 SHOWN

BRIDGE UNIT 2 SIMILAR BUT HANDED



100 DIA R6 SPIRAL AT 100 PITCH

HD12 TOP TIES



REINFORCING SECTION AT END BLOCK OF BRIDGE UNIT 4 SHOWN

REINFORCING SECTION AT DIAPHRAGM OF BRIDGE UNIT 4 SIMILAR

BRIDGE UNIT 1 SIMILAR BUT HANDED

NOTE:
-ALL DIMENSIONS AND LEVELS ARE TO BE CHECKED ON SITE PRIOR TO FABRICATION.

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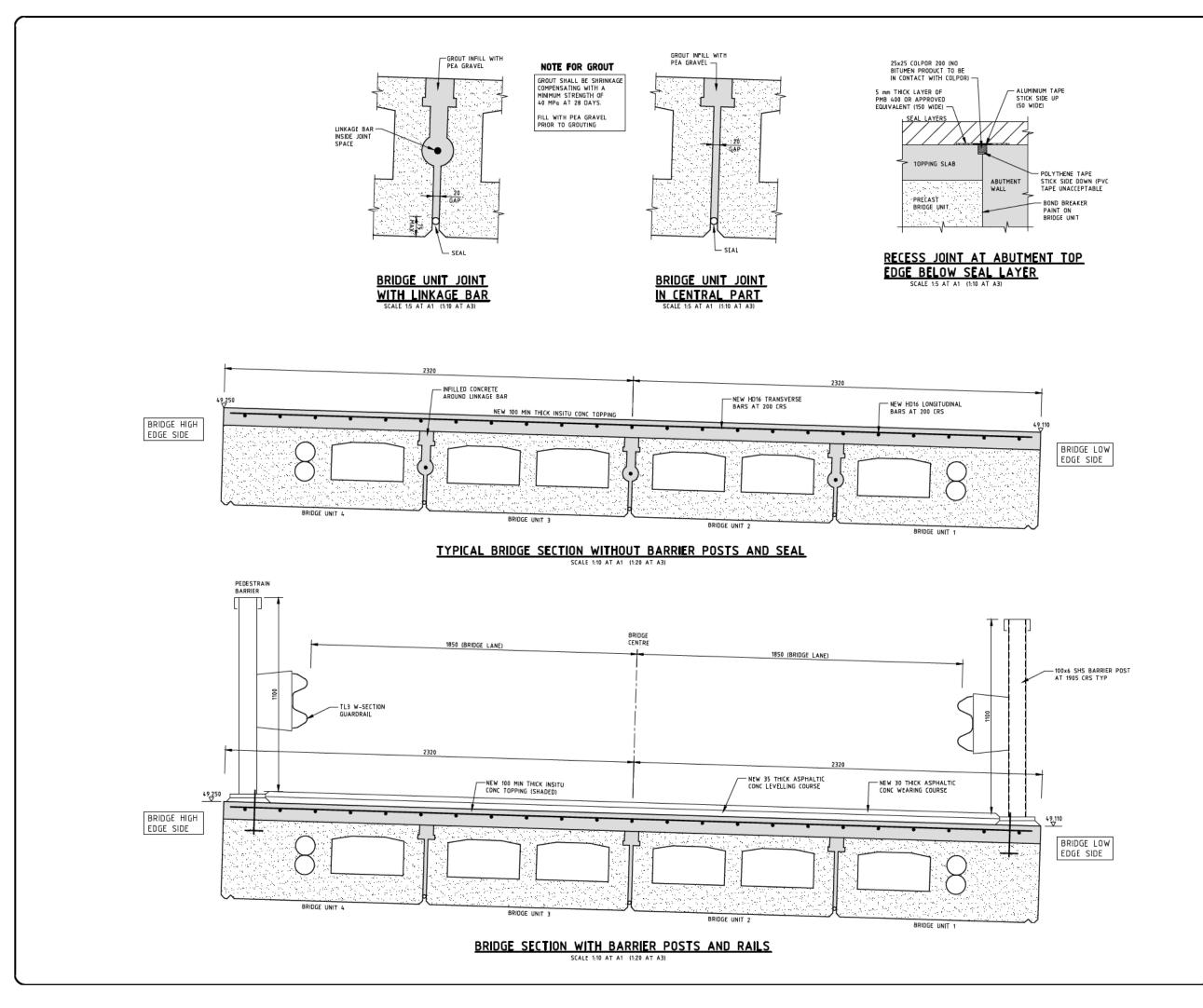
BRIDGE No.6 O/S 330 SOUTH KARORI ROAD, WELLINGTON

DRAWING TITLE

NEW BRIDGE PRECAST SECTIONS AT END BLOCK AND AT DIAPHRAGMS

| SCALE | ۸۵ | SHOWN | ۸ 1 |
|----------------------|------|-----------------------------------|-------|
| o Drawn O Checked | 88 | □ FIELD BOOK = □ DATE AUG 201: | 5 |
| □ DESIGNED | APdG | □ CAD FILE E15-0296-0 | 04+20 |
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DRAWING NUMBER
E15-0296-17



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BRIDGE No.6 O/S 330 SOUTH KARORI ROAD, WELLINGTON

DRAWING TITLE

NEW BRIDGE PRECAST DECK WITH CONC TOPPING AND BARRIER POSTS

AS SHOWN SIZE A PRAWING NUMBER REV E15-0296-18 A